



K.S. SCHOOL OF ENGINEERING AND MANAGEMENT, BANGALORE - 560109
DEPARTMENT OF MECHANICAL ENGINEERING
I SESSIONAL TEST SCHEME & SOLUTION 2022 – 23 EVEN SEMESTER
SET-A

USN								
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Degree : B. E
Branch : Mechanical Engineering
Course Title : Finite Element Method
Duration : 90 Minutes

Semester : VI
Date : 19-04-2023
Course Code : 18ME61
Max Marks : 30

Note: Answer ONE full question from each part

Q. No.	Questions with Scheme & Solution	Marks
1(a) Sol	<p>Steps in FEM</p> <ol style="list-style-type: none"> Discretization or subdivision of the domain Selection of the displacement functions Formulation of the elemental stiffness matrix Assembly of stiffness matrix Applying the boundary conditions Determining the nodal displacement Determining the elemental stress and strain <p>Explain</p>	① 2m ② ③ 2m ④ ⑤ ⑥ 1m
(b) Sol	<p>$\Pi = \frac{1}{2} \left[k_1 u_1^2 + k_2 u_2^2 + k_3 u_3^2 + k_4 u_4^2 \right] - [F_1 u_1 + F_2 u_2 + F_3 u_3 + F_4 u_4]$</p> <p>$\delta_1 = u_1, \quad \delta_2 = u_1 - u_2, \quad \delta_3 = u_3 - u_2, \quad \delta_4 = -u_4$</p> <p>$\therefore \Pi = \frac{1}{2} \left[k_1 u_1^2 + k_2 (u_1 - u_2)^2 + k_3 (u_3 - u_2)^2 + k_4 u_4^2 \right] - [F_1 u_1 + F_2 u_2 + F_3 u_3 + F_4 u_4]$</p> <p>$\frac{\partial \Pi}{\partial u_1} = 0, \quad \frac{\partial \Pi}{\partial u_2} = 0, \quad \frac{\partial \Pi}{\partial u_3} = 0. \quad \rightarrow \textcircled{1}$</p> <p>Solve all the equations</p> <p>$u_1 = 2.86 \text{m}$ $u_2 = 1.81 \text{m}$ $u_3 = 1.05 \text{m}$</p> <p style="text-align: right;">} → $\textcircled{2}$</p>	eqn-1 2m eqn-2 eqn-3 = 3m eqn-4 1m

2(a)
Sol

Node numbering scheme

<table border="1" style="border-collapse: collapse; width: 100px; height: 100px;"> <tr><td>1</td><td>2</td><td>3</td><td>4</td></tr> <tr><td>5</td><td>6</td><td>7</td><td>8</td></tr> <tr><td>z</td><td>z</td><td>z</td><td>z</td></tr> <tr><td>77</td><td>78</td><td>79</td><td>80</td></tr> </table> B=15 (a) along the shorter dimension	1	2	3	4	5	6	7	8	z	z	z	z	77	78	79	80	<table border="1" style="border-collapse: collapse; width: 100px; height: 100px;"> <tr><td>1</td><td>21</td><td>41</td><td>61</td></tr> <tr><td>2</td><td>22</td><td>42</td><td>62</td></tr> <tr><td>3</td><td>23</td><td>43</td><td>63</td></tr> <tr><td>z</td><td>z</td><td>z</td><td>z</td></tr> <tr><td>20</td><td>40</td><td>60</td><td>80</td></tr> </table> B=63 (b) along the longer dimension	1	21	41	61	2	22	42	62	3	23	43	63	z	z	z	z	20	40	60	80
1	2	3	4																																		
5	6	7	8																																		
z	z	z	z																																		
77	78	79	80																																		
1	21	41	61																																		
2	22	42	62																																		
3	23	43	63																																		
z	z	z	z																																		
20	40	60	80																																		

Since most of the matrices involved in the finite element analysis are symmetric and banded. The required computer storage can be considerably reduced by storing only the elements involved in half bandwidth instead of storing the whole matrix. The bandwidth of the assemblage matrix depends on the node numbering scheme and the number of degrees of freedom considered per node. If we can minimize the bandwidth, the storage requirements, as well as solution time can also be minimized

3m

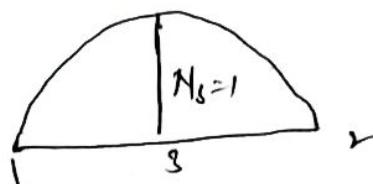
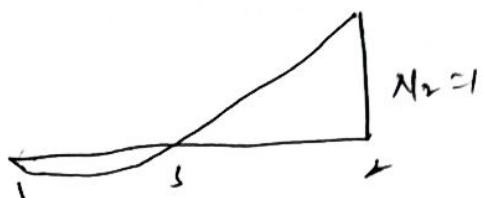
(c)
Sol

Consider a bar element in natural coordinate system.

$$X_{11} = \frac{f}{2} (f-1)$$

$$X_{22} = \frac{f}{2} (f+1)$$

$$X_{33} = 1 - f$$



2m

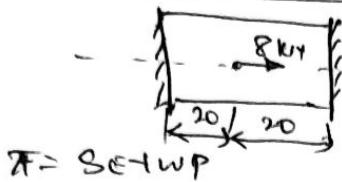
2m

1m

OR

2m

(b)
Sol



$$\Pi = \frac{EA}{2} \int \left(\frac{\partial u}{\partial x} \right)^2 dx - P u_1$$

Displacement function

$$u = a_0 + a_1 x + a_2 x^2$$

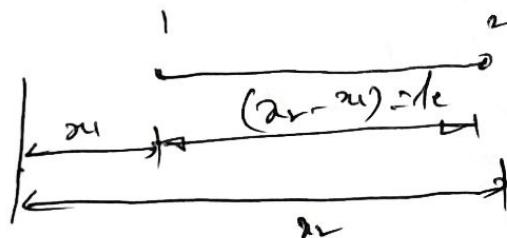
Applying BC's and Simplify

$$\Pi = \frac{EA}{2} \int (4a_2^2 x^2 + 16a_2 a_1 x + 16a_1^2) dx + 8 \times 10^{-3} a_1 \times 8000 a_2 \quad \text{--- (1)}$$

$$\frac{\partial \Pi}{\partial a_2} = 0, \quad a_2 = -0.01 \times 10^{-3} \quad \text{--- (2)}$$

$$\therefore u_1 = 0.00857 \text{ mm} \quad \text{--- (3)}$$

$$\text{at } x=20, \quad \delta=0, \quad \theta=60 \text{ N/mm}^2, \quad r=-60 \text{ N/mm}^2 \quad \text{--- (4)}$$



u_1, u_2 = Displacements at node 1 & node 2

$N_{1,2}$ = Shape functions at node 1 & node 2
shown polynomial functions:-

(c)
Sol

$$u = a_0 + a_1 x \quad \text{--- (1)}$$

After Simplification

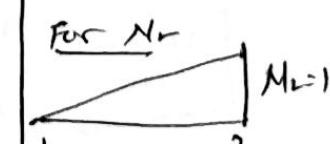
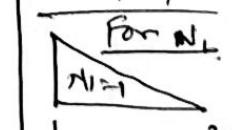
$$a_1 = \frac{u_2 - u_1}{x_2 - x_1} \quad \text{--- (2)}$$

$$a_0 = \frac{u_1 x_2 - u_2 x_1}{x_2 - x_1}$$

$$u = \frac{(x_2 - x)}{(x_2 - x_1)} u_1 + \frac{(x - x_1)}{(x_2 - x_1)} u_2$$

$$N_1 = \left(\frac{x_2 - x}{x_2 - x_1} \right), \quad N_2 = \left(\frac{x - x_1}{x_2 - x_1} \right) \quad \text{--- (3)}$$

Variations :-



PART-B

The finite element is a numerical technique used to obtain the approximate solution as the element size is reduced. This sequence of approximate solution is still converge to the exact solution, if the interpolation model satisfy the following conditions called convergence

1. The displacement field must be continuous
2. The displacement must be compatible between adjacent element
3. The displacement field must represent constant strain rates of the element
4. The displacement function must represent the rigid body displacement of the element
5. The displacement function must have the no of generalized co-ordinates equal to the nodal displacement or degree of freedom of the element
6. Spatial isotropy

Explain

Each
point
carries
1 n

3(a)
Sol

$$\Pi = S.E + w.P$$

$$\Pi = \frac{EI}{2} \int \left(\frac{dy}{dx} \right)^2 dx - \int P_0 y dx$$

Assume displacement function:-

$$y = a_0 + a_1 x + a_2 x^2 + a_3 x^3$$

Applying B.C.

$$y = a_2 x^2 + a_3 x^3$$

$$\therefore \Pi = \frac{EI}{2} \left[\frac{d^2y}{dx^2} 4a_2 L + 12a_3 L^3 + 12a_2 a_3 L^2 \right] - P_0 \left[a_2 \frac{L^3}{3} + a_3 \frac{L^4}{4} \right] \quad \text{--- (1)}$$

$$\frac{\partial \Pi}{\partial a_2} = 0, \quad \frac{\partial \Pi}{\partial a_3} = 0. \quad \text{--- (2)}$$

$$a_2 = \frac{5PL^3}{24EI}$$

$$a_3 = -\frac{PL}{12EI}$$

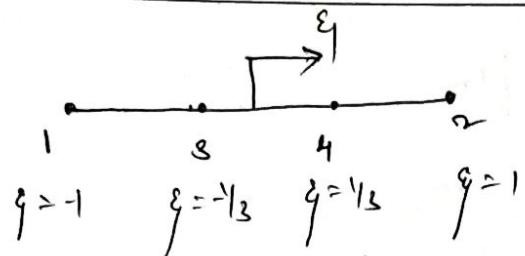
$$\therefore y_{max} = \frac{PL^4}{8EI}$$

(b)
Sol

cum 0.22m

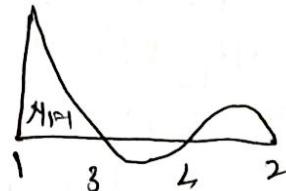
20W0 = 2m

20W0 = 1m



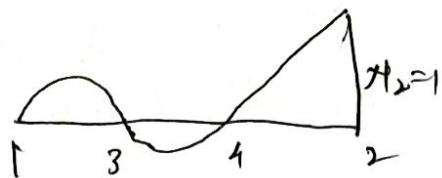
Consider a cubic bar element in natural coordinate system.

$$N_1 = -\frac{q}{16} \left(\epsilon^2 - \frac{1}{q} \right) (\epsilon - 1)$$

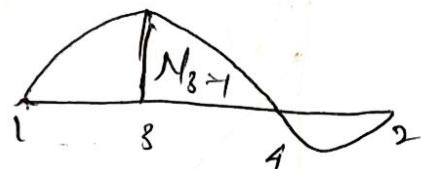


(c)
Sol

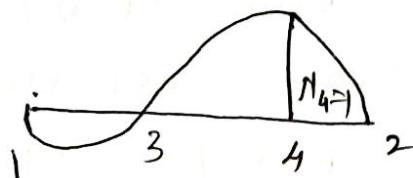
$$N_2 = \frac{q}{16} \left(\epsilon^2 - \frac{1}{q} \right) (\epsilon + 1)$$



$$N_3 = \frac{27}{16} \left(\epsilon^2 - \frac{1}{q} \right) (\epsilon - 1/3)$$



$$N_4 = -\frac{27}{16} \left(\epsilon^2 - \frac{1}{q} \right) (\epsilon + 1/3)$$



OR

Variational Principles in FEM

1. Principle of virtual work

A body is in equilibrium, when the internal virtual work equal to the external virtual work

2. Principle of virtual displacement

Are those satisfy the boundary conditions and ensures that no discontinuities such as voids

3. Potential energy

It is defined as Sum strain energy due to internal stresses and work potential due to external force

4. Minimum Potential energy Principle

It states that partial differentiation of potential energy with respect to variables is always equal to zero

Explain

(b)
Sol

$F = S + w P$

$\tau = \frac{EI}{2} \left(\frac{dy}{dx} \right) dx - P y_{max}$

Assume displacement function

$y = a_0 + a_1 x + a_2 x^2 + a_3 x^3$

Apply BC & dim. fn.

$y = a_0 + a_1 x + a_2 x^2$

i) $\tau = EI [2a_1 + 6a_2 x^2 + 6a_3 x^3] - P [a_0 + a_1 x^2]$ eqn ①

$\frac{\partial \tau}{\partial a_1} = 0 \quad \frac{\partial \tau}{\partial a_2} = 0$

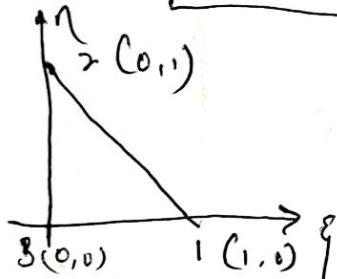
$Ta_1 = \frac{Pc}{2EI}$

$Ta_2 = -\frac{P}{6EI}$

$y_{max} = \frac{Pc^3}{3EI}$

- ④

- ⑤



Consider a CST element in natural coordinate system 2m

To derive the shape function the basic equation is

$$N = a_1 + a_2 y + a_3 y^2$$

Shape function at node 1

$$N_1 = 1$$

Shape function at node 2

$$N_2 = y$$

Shape function at node 3

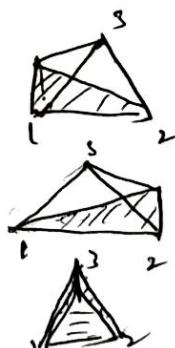
$$N_3 = 1 - y - 1$$

Explain

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1m

1m

1m



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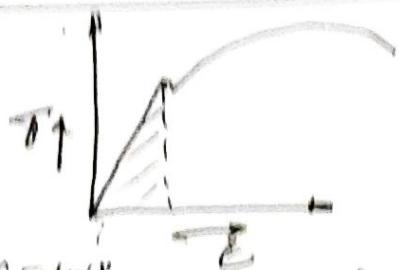
Degree : B. E
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Note: Answer ONE full question from each part

Q. No.	Questions with Scheme & Solution	Marks
	PART-A	
1(a) Sol	<p>Discretization Process Dividing the continuum into number of elements is called Discretization, This is equivalent to replacing the domain having an infinite number of degrees of freedom by a system having finite number of degrees of freedom the basic requirements during Discretization are</p> <p>a) Type of elements b) Size of elements c) Location of nodes d) Number of element e) Node numbering scheme f) Automatic mesh generation</p> <p>Explain</p> $x = s(t) - w_1^*$ $\ddot{x} = \frac{1}{2} [k_1 u_1^* + k_2 u_2^* + t_2 (u_2 - u_1)^2] - [f_1 u_1 + f_2 u_2] \quad \text{--- (1)}$ $\frac{\partial \ddot{x}}{\partial u_1} = 0 \quad \frac{\partial \ddot{x}}{\partial u_2} = 0 \quad \rightarrow \text{--- (2)}$ <div style="border: 1px solid black; padding: 5px;"> $u_1 = 1.1m$ $u_2 = 1.725m$ </div> --- (3)	<p>1 m</p> <p>a, b, d, e = 2 m</p> <p>c, f = 2 m</p>
(b) Sol	$x = s(t) - w_1^*$ $\ddot{x} = \frac{1}{2} [k_1 u_1^* + k_2 u_2^* + t_2 (u_2 - u_1)^2] - [f_1 u_1 + f_2 u_2] \quad \text{--- (1)}$ $\frac{\partial \ddot{x}}{\partial u_1} = 0 \quad \frac{\partial \ddot{x}}{\partial u_2} = 0 \quad \rightarrow \text{--- (2)}$ <div style="border: 1px solid black; padding: 5px;"> $u_1 = 1.1m$ $u_2 = 1.725m$ </div> --- (3)	<p>$\text{--- (1)} = 2 m$</p> <p>$\text{--- (2)} = 2 m$</p> <p>$\text{--- (3)} = 1 m$</p>

PART-B



$E = \int \sigma d\varepsilon + w_p$

Strain energy can be found by the area under the curve

3(a)
Sol

$$\therefore \sigma = \frac{dE}{d\varepsilon} \propto \varepsilon \quad \text{---(1)}$$

$$w_p = w_p^f + w_p^q + w_{\text{potential}}^p \quad \text{---(2)}$$

$$w_p = \int v_f^f d\varepsilon + \int v_T^q ds + \sum v_i^p p_i$$

$$E = \frac{1}{2} \int \sigma \varepsilon d\varepsilon = \left[\int v_f^f d\varepsilon + \int v_T^q ds + \sum v_i^p p_i \right] \quad \text{---(3)}$$

1m

$\Rightarrow \frac{\partial E}{\partial \varepsilon} = 2m$

$\Rightarrow \frac{\partial E}{\partial \varepsilon} = 2m$



1m

$E = \int \sigma \varepsilon d\varepsilon$

$$E = \frac{C_1}{2} \left[4a_1 L + 12a_2 L^3 + 12a_3 L^5 \right] - P_0 \left[\frac{a_1 b^3}{3} + \frac{a_2 L^4}{4} \right] \quad \text{---(1)}$$

1m

(b)
Sol

$$\frac{\partial E}{\partial a_1} = 0, \quad \frac{\partial E}{\partial a_2} = 0 \quad \text{---(2)}$$

$$\boxed{a_1 = \frac{3+L^3}{2mn^2}} \quad \text{---(3)}$$

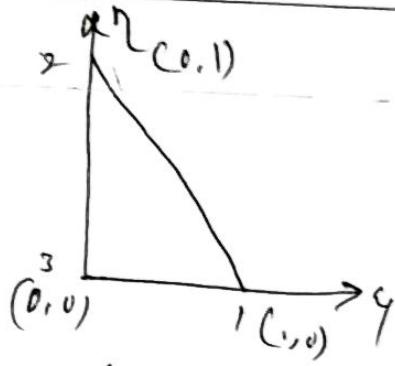
$$\boxed{a_2 = \frac{PL}{12mn^2}} \quad \text{---(4)}$$

$$\boxed{y_{\text{min}} = \frac{PL^3}{8n^2}} \quad \text{---(5)}$$

1m

1m

1m



Shape function at node 1

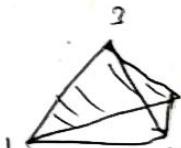
$$N_1 = \xi$$

Shape function at Node 2

$$N_2 = \eta$$

Shape function at nodes

$$N_3 = (1 - \xi - \eta)$$



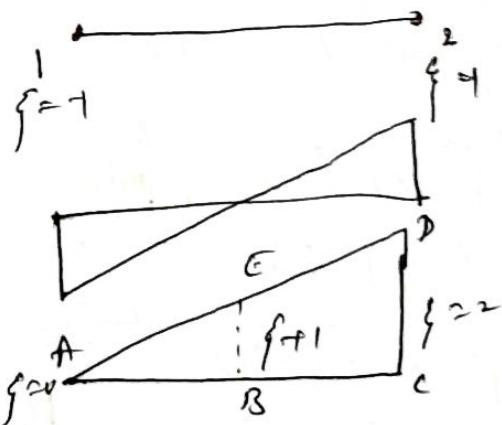
1m

1m

1m

2m

OR



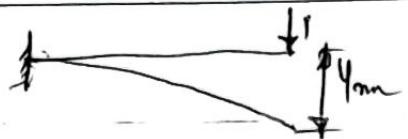
from similar triangle

$$\frac{AB}{AC} = \frac{BF}{CE}$$

$$\frac{f}{g} = \frac{2(n-2h)}{(2n-2h)} - 1$$

3m

2m



$$\Delta I = \delta I - Iw^2$$

$$I = \frac{EI}{2} \int \left(\frac{dy}{dx} \right)^2 dx - Pw$$

Assuming displacement function

$$y = a_0 + a_1 x + a_2 x^2 + a_3 x^3$$

Applying BC's

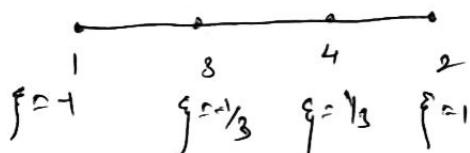
$$y = a_2 x^2 + a_3 x^3$$

$$\therefore I = EI \left[2a_2 L + 6a_3 L^2 + 6a_2 a_3 L \right] - P \left[a_2 L^2 + a_3 L^3 \right] \quad \text{--- (1)}$$

$$\frac{\partial I}{\partial a_2} = 0, \quad \frac{\partial I}{\partial a_3} = 0 \quad \text{--- (2)}$$

$$\boxed{a_2 = \frac{PL}{8EI}} \quad \boxed{a_3 = \frac{P}{6EI}}$$

$$\therefore w_{max} = \frac{PL^3}{8EI} \quad \text{--- (3)}$$



Consider a bar cubic bar element in natural coordinate system

(c)
Sol

$$N_{11} = -\frac{q}{16} \left(q - \frac{1}{3} \right) \left(q + 1 \right)$$



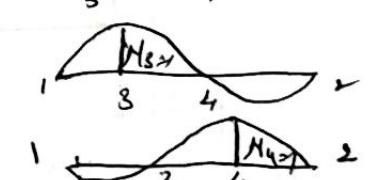
1m

$$N_{12} = \frac{q}{16} \left(q^2 - \frac{1}{9} \right) \left(q + 1 \right)$$



1m

$$N_{13} = \frac{27}{16} \left(q^2 - 1 \right) \left(q - \frac{1}{3} \right)$$



1m

$$N_{14} = -\frac{27}{16} \left(q^2 - 1 \right) \left(q + \frac{1}{3} \right)$$



1m


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